

The Examination of Effect of Salt Water on Mechanical Properties Clay Soil Around the Dam Gotvand Using Response Surface Method (RSM)

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ABSTRACT

Specifying the mechanical characteristics of soil is as one of the major steps in designing the foundation for civil projects, particularly hydraulic structures. This study examines the water salinity effect on engineering characteristics of fine-grained soil in clayey core of Gotvand dam located on the Karun River in Khuzestan province, Iran. For this purpose, three types of salt; sodium chloride, magnesium chloride and calcium sulfate is considered that the last one has the highest concentration in the water of reservoir behind the dam, and then their effect on the mechanical characteristics of clay including density, angle of internal friction, adhesion and open inflation have examined. Therefore, treatments required for testing were provided by combining the mentioned salts considering the weight percentage of salt in the water behind the dam and designing a test using response surface methodology. The results were statistically analyzed by RSM. Results showed that type and amount of soil salinity has no significant impact on the compaction characteristics of soil namely optimum moisture content and the maximum dry density. Eventually, results of open inflation showed that whatever the salt concentration be higher, the rate of sample's open inflation will be lower. Results of changes in the angle of internal friction indicate that at first this parameter decreases and then it increases. Studies on the soil adhesion show that at first this parameter increases but eventually it decreases. In the open inflation test, whatever the salt concentration is higher, the amount of sample's open inflation will be lower

KEY WORDS: Gotvand dam, Water salinity and soil, Density characteristics, Triaxial test, RSM.

INTRODUCTION

Engineering characteristics of fine-grained soils especially clayey soils are subject to several factors including density, porosity, structure, history of tension, granulation, amount and type of clayey minerals, plastic property and amount and kind of minerals in the pore water [1]. Any change in each of these characteristics will change the physical and mechanical characteristics of soil, and consequently structures which are built on soil will be exposed to change or threat. Density, shear strength and plasticity are considered as characteristics of soil engineering which play an important role in designing foundation of most civil structures [2]. Basically, theories of soil mechanics are based on the concept of effective stresses. In these theories the pore water and solid particles are both assumed to be chemically inert, so no chemical interaction takes place between the different phases. While, due to the negative charge on the surface of clayey mineral particles, electro-chemical forces exist between the solid, liquid and dissolved phases that they can mainly change the mechanical behavior of clayey soil [3]. The quantity and nature of electro-chemical forces and consequently changes in properties of soil depends on the amount and type of anion and cation in exchange phase and their impact on the cation exchange capacity [4]. Any change in soil engineering properties can cause changes in the parameters used in designing soil structures, civil and water related constructions and other structures' foundations. So, determining the behavior and reaction of soil under the influence of solutions and different materials can be effective in designing and making the correct decisions. Type of chemicals that are combined with clayey soil and also its constituents are considered as one of the important issues to understand changes in soil engineering properties [5]. This study has examined Gotvand dam as the case study. Upper Gotvand dam is one of Iran's largest dams which it is built on Karun River, southwest of Iran. This dam is located at the distance of 380 km from the outfall of the Karun river, at the distance of 25 km from north of Shushtar city and at the distance of 10 km from northeast of Gotvand city in Khuzestan province. It is the last dam which can be constructed on the Karun River. This dam's lake has a reservoir of 4.5 billion cubic meters which makes it the second largest artificial lake in Iran after Karkheh dam. The Gotvand dam is a rock fill dam with clay core and a height of 182 meters which makes it the largest earthen dam of the country. Executive operations of this project have begun in 1976. This dam is located more downstream among the built dams and dams that are currently under construction on the Karun River.

The salt domes and veins around the inundation area of dam is one of the major challenges about the dam which it goes under water after inundation and leads to excessive salinity of water in downstream of this dam. Experts of environment and engineering council believe that closeness of salt mine and Gachsaran's salt formation to the site of the Upper Gotvand dam is not considered in this research project. This mine is located 5 km away from the dam and its salt

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has been estimated hundreds of million tons. During inundation and formation of the lake behind the dam, this mine has gone completely under water and caused that the salinity of Karun River's water reaches to the highest possible extent. The passage of time proved that inundation of this dam has led to the salinity of Karun River about 10 times. After the construction operation and before beginning the inundation, constructors proceed to build a clay blanket on the salt formations. But, signs of cracking were observed on clay blanket just three days after the edges of these blankets went under water. Investigating the water behind the dam showed that the blankets had no effect on reducing the amount of water salinity. Figure 1 shows an overview of the dam.

Considering the current situation, it seems that the following problems are predictable in this dam:

1. Extreme ruin and change in the quality of water in the dam's lake
2. The instability of Gachsaran's formation and creating large landslides in the dam's reservoir
3. The damaging effects of water salinity on the dam's body in long-term



Figure 1.A view of Gotvand

This study investigates the damaging effects of water salinity on the dam's body and the clayey lands around the lake, to investigate the possibility of slippage, by studying the impact of pore water salinity on the mechanical properties of clayey soil. Various researches have conducted scattered and diverse researches on investigating and determining the effect of different salts and solutions on the engineering properties of clayey soils. Lambe[4]states that in a certain porous ratio, any change in the water-soil system that inflates the dual layer will reduce the resistance. Mitchell [5]believes that in addition to the type and amount of clay in the soil, its chemical characteristics such as the amount and type of anion and the existing cations have a substantial impact on the physical and mechanical behavior of soil such as construction, aggregate instability, consolidation and especially inflation. Abdullah et al.,[6] have investigated the impact of sodium, calcium and potassium on the consolidation characteristics of clayey soils. Paassen and Gareau [7] have examined the impact of pore water salinity on the clayey soil compaction. Yilmaz[8] showed that there is an appropriate relationship between cation exchange capacity and mechanical properties such as inflation and liquidlimit. Yukselen et al.,[9]claim that soils engineering behavior varies depending on the type of clay minerals. So that, properties of Montmorillonite clays strongly change against the sea water. Rao and Thyagaraj[10] investigated the effect of sodium chloride solution on inflation behavior of compacted clayey soil by using consolidation test. Baumgartner et al.,[11] suggested that calcium chloride reduces the potential for inflation. Olson and Mitronovas[12] figured out that the electrolyte concentration has little effect on the Atterberg limits by examining the consolidation characteristics and shear strength of calcium and magnesium illites. The study results from Kenny [13] suggest that the resistance of remaining clays depends on the type of minerals and the chemical properties of pore water. Chatopadhaya[14] measured the shear strength remaining from samples of Montmorillonite clays that had been affected by salt water and he showed that the remaining shear strength will rise by increasing the concentration of water. Maio[15] also finds similar results and conducted some tests on the liquid limit of disturbed samples of Bentonite with NaCl and he observed that liquid limit will decrease rapidly by increasing the salt concentration. Mashkouh[16] examined the impact of salt and moisture on mechanical properties of clayey soil. Abbasi[17] also investigated the effect of pore water salinity on the consolidation and compaction properties of clayey soil. This study also investigates the effect of pore water salinity on some geotechnical characteristics of clayey soil by considering the abundant use of clayey soils in the foundations' structures, due to the high compressibility, and also the main body of embankment dams and high level of water salinity in the vast areas of the world, including Iran.

MATERIAL AND METHODS

This study is used samples with different salinity level to investigate the effect of pore water salinity on geotechnical characteristics of clayey soil. For this purpose, at first a sample of clayey soil with the physical properties in table 1 is prepared from Emam–zadehZeid borrow source near the Gotvand dam. Then, the experimental treatments were made by adding different amounts of salt, sodium chloride, magnesium chloride and calcium sulfate. After preparing the sample of natural soil, different amounts of ionic composition containing the mentioned salts was added to the samples in the ratio of zero to eleven percent by weight in order to obtain a sample with the desired chemical characteristics. Then, the test designed by using RSM.

Finally, the results of measurements were analyzed statistically using RSM and final versions and graphs were extracted from this software. Table 1 shows the characteristics of salts used in this project.

Table 1. Characteristics of salts used in this project

Salt	Molecular formula	Appearance	Molar mass	Density	Melting temperature (°C)	Solubility in 20° water
Sodium chloride	NaCl	Colorless or white crystal	58.44	2.16	801	35.9
Magnesium chloride	MgCl ₂	Colorless or white crystal	95.211	2.32	714	54.3
Calcium chloride	CaSO ₄	White solid	136.14	2.96	1460	0.21

At first, the percentage of salt used in the new salt composition was determined according to the water behind the dam in order to build the samples. Salts were combined according to the specified weight percentage. The new salt composition was combined with the dry soil completely homogenous by using the software according to the specified weight percentage; then, the necessity moisture content was added to the samples. Table 2 gives the characteristics of percentage of salts used in samples according to renewed designing the software.

The ratio of sodium chloride, magnesium chloride and calcium sulfate percentage to the total salt is considered with a little overlook, respectively 50, 30 and 20.

Table 2. Characteristics of percentage of salts used in samples according to renewed designing the software

Designed tests	Weight percentage of ionic composition to the soil	Weight percentage of Nacl to the soil	Weight percentage of Mgcl2 to the soil	Weight percentage of Caso4 to the soil
1	5.5	2.75	1.65	1.1
2	8.25	4.125	2.475	1.65
3	0	0	0	0
4	2.75	1.375	0.825	0.55
5	11	5.5	3.3	2.2
6	0	0	0	0
7	11	5.5	3.3	2.2

Diagnostic Tests

Initial tests including granulation, determining Atterberge limits, dry density and also extracting chemical properties of soil were done according to ASTM in order to identify and determine the physical characteristics of used natural soil. Table 3 shows the physical and chemical characteristics of used soil.

Table 3. Characteristics of Initial Soil

Soil type	Passing percentage of 4.76 mm	Passing percentage of 0.076 mm	Passing percentage of 0.002 mm	Dry density	Moisture	Liquid limit	Plastic limit	Plastic index	PH
CL	100	95	25	1.29	4.55	55	18	37	7.4

Open inflation test

Inflation test was carried out based on ASTMD4546 standard and through method (A). In this method, the amount of soil sample inflation under the initial overload of apparatus, 1kPa, was measured over time. Parameters measured in this test have a high percentage.

Density Testing

Compaction characteristics of soils including dry unit weight and optimum moisture are as the important characteristics of soil in engineering issues such as the constructing embankments related to the paths of channels, roads, bodies of dams and other similar cases. These characteristics can be determined by testing density of sample. This research is used standard proctor test provided in ASTM for determining the characteristics. So that, sample was compacted in three layers and each layer with 25 beats with various moisture content.

Ttriaxial test

It is down in accordance with ASTM

Designing the Test

Designing the test was done with regard to the two factors, moisture content and salt, to test the open inflation by using RSM. Figure 2 shows designing the test by software for Open Inflation Test.

Std	Run	Block	Factor 1 A:salt %	Factor 2 B:moist %	Response c kPa	Response Q c	Response sw %
4	1	Block 1	9.39	22.07	31	10	6.1
3	2	Block 1	1.61	22.07	84.5	6	9.7
1	3	Block 1	1.61	7.93	94	12	21.6
2	4	Block 1	9.39	7.93	35	12	15.9
7	5	Block 1	5.50	5.00	118.1	3.5	18.5
5	6	Block 1	0.00	15.00	52.67	36	14.4
6	7	Block 1	11.00	15.00	34.33	13	4.8
8	8	Block 1	5.50	25.00	102.3	2.7	7.5

Figure 2. Designing the test with regard to the two factors of salt concentration and moisture

Std	Run	Block	Factor 1 A:A	Response 1 c kPa	Response 2 Q c	Response 3 yd g/cm3	Response 4 wopt %
7	1	Block 1	5.50	121.38	4	1.43	11.9
4	2	Block 1	8.25	40	13	1.42	12.2
1	3	Block 1	0.00	59.67	38	1.33	12.3
3	4	Block 1	2.75	105	9	1.4	11.9
6	5	Block 1	11.00	30	15	1.45	11.8
2	6	Block 1	0.00	60	37	1.5	12
5	7	Block 1	11.00	32	15	1.37	11.6

Figure 3. Redesigning the test based on the one factor model

Also, figure 3 shows designing the software triaxial test Limits and density considering the factor of increasing concentration.

RESULTS AND DISCUSSION

After running the relevant tests on the samples in question, characteristics of each treatment was determined, then the role of chemical quality of samples on the noted characteristics was analyzed.

Designing the test for open inflation was done with regard to the two factors of moisture and salt, and this test's results were analyzed.

Open Inflation Test

The results of this test are presented in the column Sw of figure 2. Data analysis was done by RSM. Figure 4 shows the simultaneous effect of increasing the percentage of salt and moisture.

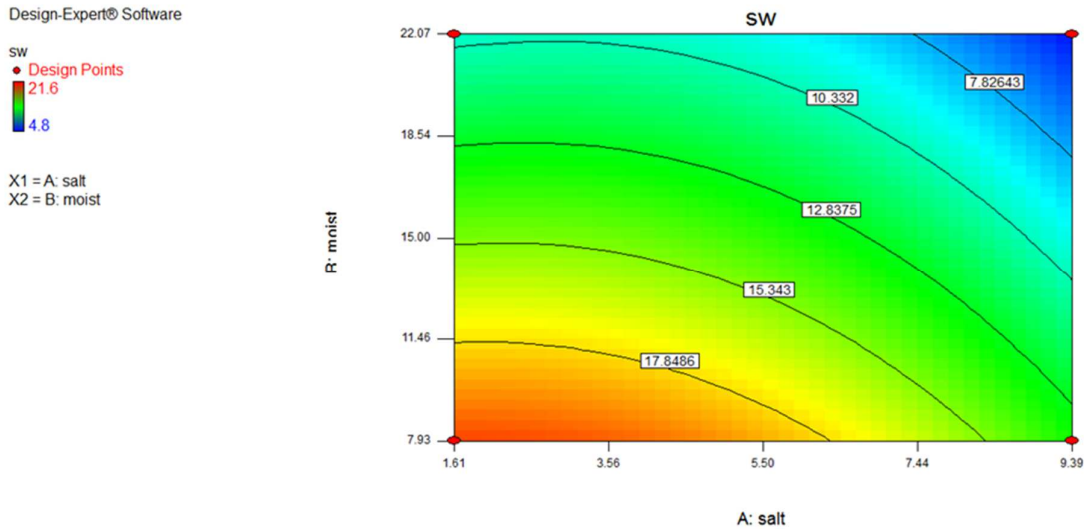


Figure 4. The simultaneous effect of increasing the percentage of salt and moisture on open inflation

As it can be seen in figure 4, whatever the salt concentration is higher, the rate of sample's open inflation will be lower. This reduction in open inflation percentage applies in all points of the curve with different moistures percentage.

The percent of open inflation is reduced by increasing the percent of moisture. According to the preliminary tests, the optimum soil moisture content is about 12 percent. According to the level of response presented in figure 10, it can be seen that samples with dryer moisture content from optimum moisture have more inflation than more wet points from optimum point, and this matter is apart from the percent of salt.

Also the samples which were exposed to the higher salinity shows less sensitivity to increasing the moisture content.

Inflationary behavior of the soil in contact with salt waters is mainly analyzed with regard to factors: disseminating saline minerals into the soil and reducing the double layer thickness of soil.

Figure 5 and 6 shows the minimum and maximum open inflation mode, respectively. According to the figures, the lowest inflation occurs when the percent of moisture and salt is at the highest level and the highest percent of open inflation occurs when the percent of moisture and salt concentration is at the lowest level.

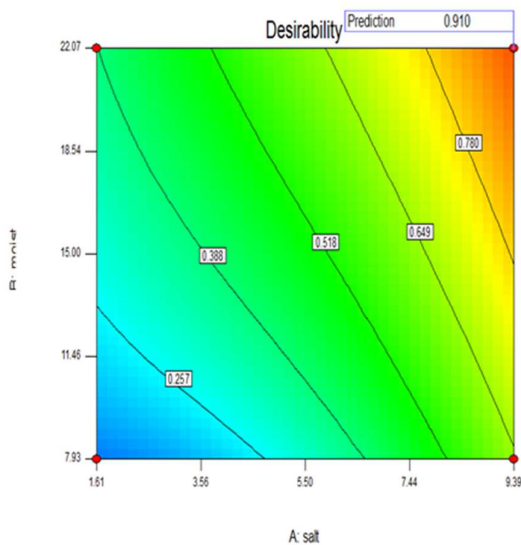


Figure 5. The lowest percent of open inflation

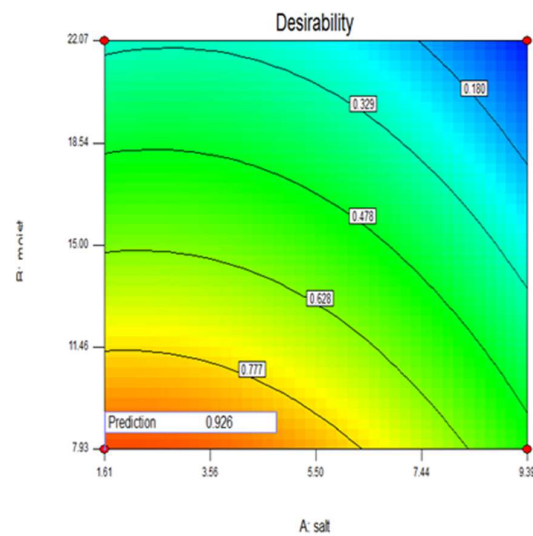


Figure 6. The highest percent of open inflation

Density Test

Density curve was drawn for each sample by implementing density standard proctor test on different treatments and the compaction characteristics means the maximum dry unit weight and the optimum moisture of

all tested sample were determined by using these curves. Results shows that the optimum moisture varies between 11.5 to 12.5 percent for all amounts of used concentrations. While, the tolerance of 2% are permitted in all projects related to the optimum moisture content. So, it can be conclude that the impact of various types and accounts of salts used in the optimum moisture content is low and negligible. Also, the scope of changes in the maximum dry unit weight obtained for all types of samples was between 1.3 to 1.5 g/cm³. It means that the difference between the highest and lowest is less than 5 percent. This amount is also less than the tolerance amount allowed for the maximum dry unit weight. Therefore, it can be concluded that the impact of noted salts on the maximum dry unit weight is also not remarkable. In general, it can be said that the amount and type of soil salinity has no significant impact on the compaction characteristics of soil. Figures 7 and 8 show the outputs related to the software.

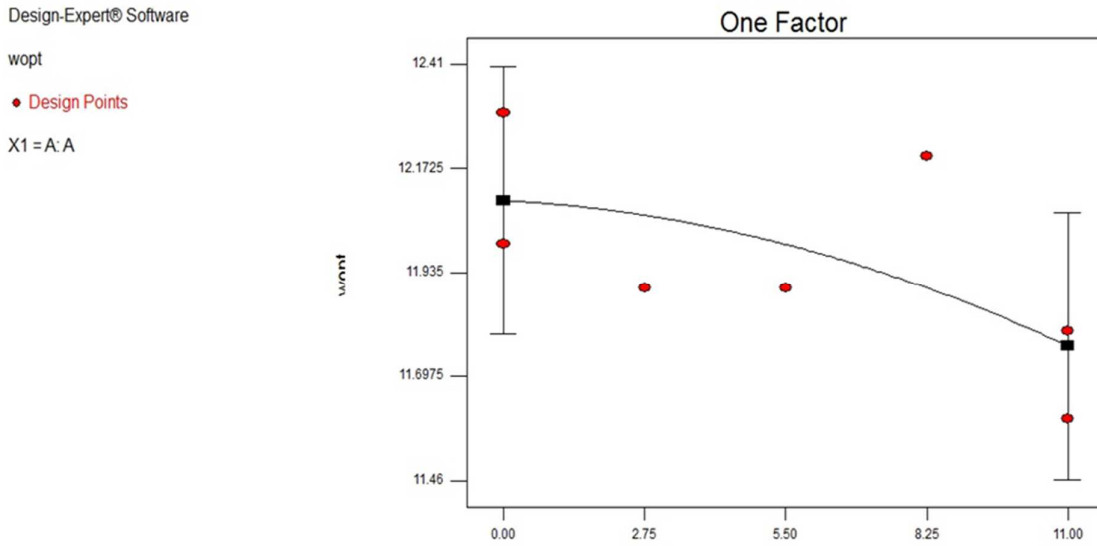


Figure 7. Diagram for changes in optimum moisture by increasing the percentage of salt

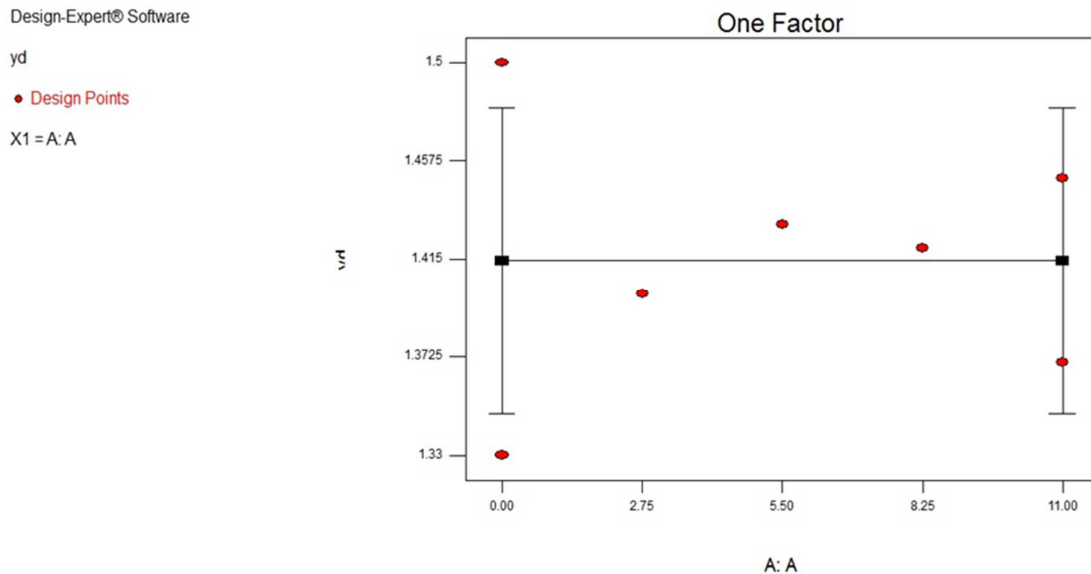


Figure 8. Diagram for changes in dry density by increasing the percentage of salt open inflation will be lower.

The Triaxial Test

Adhesion and the angle of internal friction columns in figure 2 present the results of this test. Figure 9 suggests the changes in soil adhesion through increasing in salt concentration and moisture. Analysis of software shows that two factors of salt concentration and moisture have no interaction and they act

independently. RSM software suggests that designing of test is considered as one factor and the model presented in this case is not acceptable. Also, colors and lines of RSM show that adhesion decreases by increasing moisture.

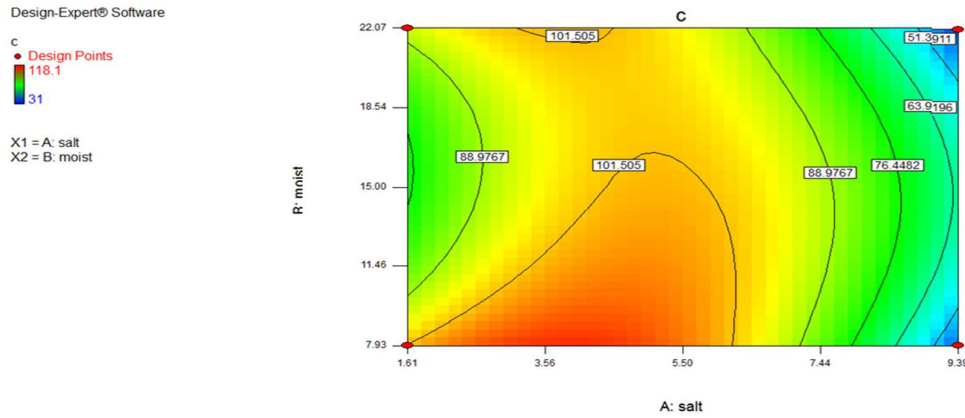


Figure 9.Changes in soil adhesion by increasing salt concentration and moisture

Figure 10 indicates changes in soil's angle of internal friction by increasing salt concentration and moisture. Analysis of software shows that two factors of salt concentration and moisture have no interaction and they act independently. Like adhesion, designing of test is considered as one factor and the model presented in this case is not acceptable. Also, colors and lines of RSM show that the angle of internal friction decreases by increasing moisture.

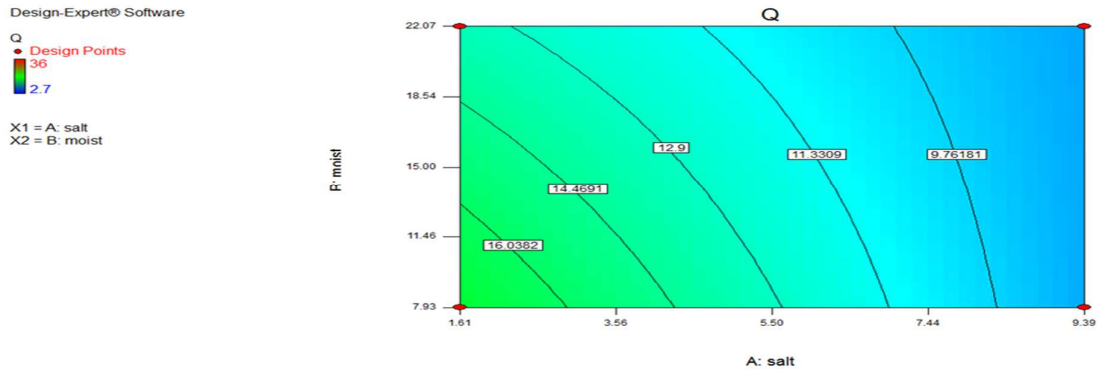


Figure 10.Changes in soil's angle of internal friction by increasing salt concentration and moisture

Here, the test is redesigned based on the one factor in order to achieve the required result. Figure 11 shows the diagram of analyzing results of triaxial test and adhesion parameter. As it can be seen in figure 16, at first soil adhesion increases by increasing percentage of salt concentration, then it decreases.

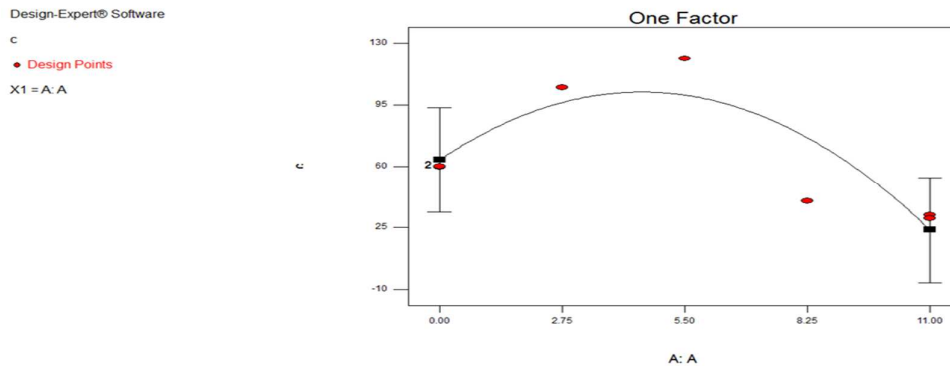


Figure 11.Changes in soil adhesion by increasing salt concentration

The following model is presented by software to predict results. Figure 12 shows the impact of increasing the percentage of salt on the soil's angle of internal friction. As it is shown in the figure, at first soil's angle of internal friction decreases by increasing the percentage of salt, and after a while it begins to increase.

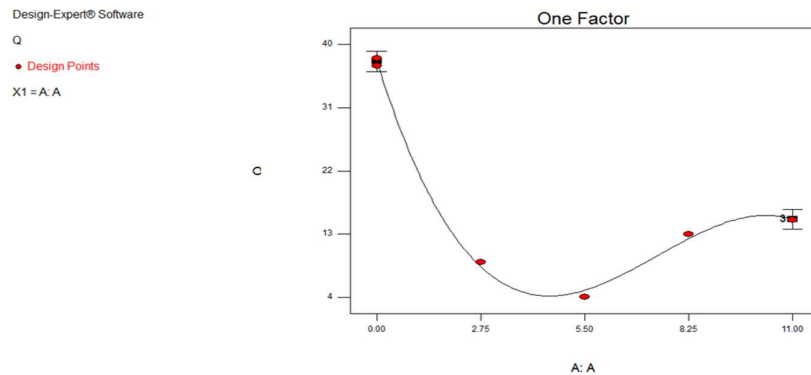


Figure 12. Changes in soil's angle of internal friction by increasing the salt concentration

The following model is presented by software to predict results. Mocher is presented a hypothesis for material rupture in 1900; it is on the basis that material is fractured in a critical combination of shear and vertical stresses, and not solely as a result of the maximum vertical stress or simply the maximum shear stress.

The rate of adhesion and angle of internal friction are the parameters for measuring the strength in the test. According to the obtained results it can be said that the rate of adhesion during testing first increases and then it decreases. Figure 11 shows the matter. The reason for this could be that soil adhesion strength expands through a negative cavity pressure in open cavity fluid. Samples' angle of friction is the most important factor to compare their strength. Figure 12 shows changes in angle of internal friction according to the percentage of salt. According to Lambe [4], any change in soil and water systems which swells the double layer also it subsequently will decrease the soil strength; the occurrence of such a phenomenon is due to the increased inter-particle range. So according to the mentioned issues and in explaining the results, it can be said that normally there is an electrostatic repulsion force between negative charges on the clay surface. Also, prevailing stresses between clay particles are repulsive stresses with high-range and in the neutral position; the double layer expands around the particles. Since, the tested soil is a kind of clay with high activity and it has a huge specific surface and a lot of not neutralized charged particles. By increasing the percentage of salt, at first the resistance of high-range charged forces will observe and it will increase the repulsive force; which subsequently shear strength of soil decreases, as well. While, thickness of double layer decreases by further increasing of salt percentage, then repulsive force also decreases and subsequently the shear strength of soil increases.

CONCLUSION

This study has investigated the effect of pore water salinity with percentage and types of salt in the reservoir's water behind the Gotvand dam on the soil used in the clayey core of the dam.

The following conclusions can be inferred and expressed based on the investigations and laboratory tests carried out in this study:

The effect of different concentrations of salt on the optimum moisture content and maximum dry density is very slight and within the allowed tolerance. It means that the amount and type of soil salinity has no significant impact on the compaction characteristics of soils.

Contact of soil with salt waters reduces the plasticity of soil along with decisive reduction in the liquid limit, but there was no difference in the plastic limit of soil.

In the open inflation test, whatever the concentration of salt is higher, the rate of sample's open inflation will be lower. This reduction in the percent of open inflation applies in all points of curve with different percent of moistures.

The percent of open inflation decreases by increasing the percent of moisture.

Regarding that the optimum soil moisture content is about 12 percent, it can be determined that samples with dryer moisture content from optimum moisture have more inflation than more wet points from optimum point, and this matter is apart from the percent of salt.

Also the samples which were exposed to the higher salinity shows less sensitivity to increasing the moisture content.

Soil behavior in contact with salt waters is mainly predicted with regard to factors: disseminating saline minerals into the soil and reducing the double layer thickness of soil.

According to the strength parameters obtained from the triaxial test, at first the angle of internal friction decreases and then it increases. In soil adhesion, this parameter first increases by increasing the salt and after a while it decreases.

Results show that salinity influences on the mechanical characteristics of tested soil. This matter has no destructive impact on the dam body at least in short-term. But, there is a need to investigate the different scenarios in various time cycles, regression periods especially when the salt concentration reduced again, and also to conduct further studies in order to achieve the final results.

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